INTERIOR WALL HEIGHTS

With structural framing

2-1/2" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
250S137-33	12	17' 6"	13' 10"	12' 1"
	16	15' 10"	12' 7"	11' 0"
	24	13' 10"	11' 0"	9' 7"
250S137-43	12	19' 0"	15' 1"	13' 2"
	16	17' 3"	13' 8"	11' 11"
	24	15' 1"	11' 11"	10' 5"
250S137-54	12	20' 3"	16' 1"	14' 1"
	16	18' 5"	14' 8"	12' 9"
	24	16' 1"	12' 9"	11' 2"
250S137-68	12	21' 8"	17' 2"	15' 0"
	16	19' 8"	15' 7"	13' 8"
	24	17' 2"	13' 8"	11' 11"
250S137-97	12	23' 8"	18' 10"	16' 5"
	16	21' 6"	17' 1"	14' 11"
	24	18' 10"	14' 11"	13' 0"
250S162-33	12	18' 4"	14' 7"	12' 9"
	16	16' 8"	13' 3"	11' 7"
	24	14' 7"	11' 7"	10' 1"
250S162-43	12	19' 11"	15' 10"	13' 10"
	16	18' 1"	14' 4"	12' 7"
	24	15' 10"	12' 7"	11' 0"
250S162-54	12	21' 4"	16' 11"	14' 9"
	16	19' 4"	15' 5"	13' 5"
	24	16' 11"	13' 5"	11' 9"
250S162-68	12	22' 9"	18' 1"	15' 9"
	16	20' 8"	16' 5"	14' 4"
	24	18' 1"	14' 4"	12' 6"
250S162-97	12	25' 0"	19' 10"	17' 4"
	16	22' 9"	18' 0"	15' 9"
	24	19' 10"	15' 9"	13' 9"

2-1/2" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
250S200-33	12	19' 4"	15' 4"	13' 5"
	16	17' 7"	13' 11"	12' 2"
	24	15' 4"	12' 2"	10' 8"
250S200-43	12	21' 1"	16' 9"	14' 8"
	16	19' 2"	15' 3"	13' 4"
	24	16' 9"	13' 4"	11' 7"
250S200-54	12	22' 7"	17' 11"	15' 8"
	16	20' 6"	16' 4"	14' 3"
	24	17' 11"	14' 3"	12' 5"
250S200-68	12	24' 2"	19' 2"	16' 9"
	16	21' 11"	17' 5"	15' 3"
	24	19' 2"	15' 3"	13' 4"
250S200-97	12	26' 7"	21' 1"	18' 5"
	16	24' 2"	19' 2"	16' 9"
	24	21' 1"	16' 9"	14' 8"
250S250-43	12	22' 4"	17' 9"	15' 6"
	16	20' 4"	16' 1"	14' 1"
	24	17' 9"	14' 1"	12' 4"
250S250-54	12	23' 11"	19' 0"	16' 7"
	16	21' 9"	17' 3"	15' 1"
	24	19' 0"	15' 1"	13' 2"
250S250-68	12	25' 8"	20' 4"	17' 9"
	16	23' 4"	18' 6"	16' 2"
	24	20' 4"	16' 2"	14' 1"
250S250-97	12	28' 4"	22' 6"	19' 8"
	16	25' 9"	20' 5"	17' 10"
	24	22' 6"	17' 10"	15' 7"

Notes:

1 Studs are checked for simple-span deflection and stress. Stress calculations are made for mid-span fully braced moment, end shear through the unperforated section and shear moment interaction through the perforated section 10" away from the end bearing.

2 A 1/3 stress increase is not used.

3 Limiting heights are based on continuous lateral support of each flange over the full height of the stud.

4 Listed limiting heights are based on steel properties only.

5 End reactions must be checked for web crippling separately.

6 Web crippling check based on 1-inch end bearing. Where limiting heights are followed by "e", web stiffeners are required.

7 Allowable moment is the lesser of local and distortional buckling. Stud distortional buckling based on an assumed $K\phi = 0$.

8 Members marked with an [1] have h/t > 200, and thus require end stiffeners.

9 Capacities are calculated according to the AISI S100-16 (2020) w/S2-20. A 1-1/2" by 4" knockout spaced no closer than 24" o.c. is assumed. (3/4" for 2-1/2" studs).

10 All values are based on Fy=33ksi for 33mil and 43mil Studs, and Fy=50ksi for 54mil, 68mil and 97mil Studs.

11 For deflection calculations, interior wall loads have been multiplied by 1.0 per AISI S240.

Complies with AISI S100-16 (2020) w/S2-20 • IBC 2021

INTERIOR WALL HEIGHTS

With structural framing

3-1/2" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
350S137-33	12	22' 7"	17' 11"	15' 8"
	16	20' 7"	16' 4"	14' 3"
	24	17' 2"	14' 3"	12' 5"
350S137-43	12	24' 7"	19' 6"	17' 1"
	16	22' 4"	17' 9"	15' 6"
	24	19' 6"	15' 6"	13' 6"
350S137-54	12	26' 4"	20' 11"	18' 3"
	16	23' 11"	19' 0"	16' 7"
	24	20' 11"	16' 7"	14' 6"
350S137-68	12	28' 2"	22' 4"	19' 6"
	16	25' 7"	20' 3"	17' 9"
	24	22' 4"	17' 9"	15' 6"
350S137-97	12	30' 11"	24' 7"	21' 5"
	16	28' 1"	22' 4"	19' 6"
	24	24' 7"	19' 6"	17' 0"
350S162-33	12	23' 9"	18' 10"	16' 5"
	16	21' 7"	17' 1"	14' 11"
	24	18' 5"	14' 11"	13' 1"
350S162-43	12	25' 10"	20' 6"	17' 11"
	16	23' 5"	18' 7"	16' 3"
	24	20' 6"	16' 3"	14' 2"
350S162-54	12	27' 8"	21' 11"	19' 2"
	16	25' 1"	19' 11"	17' 5"
	24	21' 11"	17' 5"	15' 2"
350S162-68	12	29' 7"	23' 6"	20' 6"
	16	26' 10"	21' 4"	18' 7"
	24	23' 6"	18' 7"	16' 3"
350S162-97	12	32' 7"	25' 10"	22' 7"
	16	29' 7"	23' 6"	20' 6"
	24	25' 10"	20' 6"	17' 11"

3-1/2" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
350S200-33	12	24' 11"	19' 10"	17' 4"
	16	22' 8"	18' 0"	15' 9"
	24	19' 4"	15' 9"	13' 9"
350S200-43	12	27' 3"	21' 8"	18' 11"
	16	24' 9"	19' 8"	17' 2"
	24	21' 8"	17' 2"	15' 0"
350S200-54	12	29' 3"	23' 2"	20' 3"
	16	26' 6"	21' 1"	18' 5"
	24	23' 2"	18' 5"	16' 1"
350S200-68	12	31' 3"	24' 10"	21' 8"
	16	28' 5"	22' 7"	19' 8"
	24	24' 10"	19' 8"	17' 3"
350S200-97	12	34' 7"	27' 5"	24' 0"
	16	31' 5"	24' 11"	21' 9"
	24	27' 5"	21' 9"	19' 0"
350S250-43	12	28' 9"	22' 10"	19' 11"
	16	26' 1"	20' 9"	18' 1"
	24	22' 10"	18' 1"	15' 10"
350S250-54	12	30' 9"	24' 5"	21' 4"
	16	27' 11"	22' 2"	19' 4"
	24	24' 5"	19' 4"	16' 11"
350S250-68	12	33' 1"	26' 3"	22' 11"
	16	30' 0"	23' 10"	20' 10"
	24	26' 3"	20' 10"	18' 2"
350S250-97	12	36' 7"	29' 1"	25' 5"
	16	33' 3"	26' 5"	23' 1"
	24	29' 1"	23' 1"	20' 2"

Notes:

1 Studs are checked for simple-span deflection and stress. Stress calculations are made for mid-span fully braced moment, end shear through the unperforated section and shear moment interaction through the perforated section 10" away from the end bearing.

2 A 1/3 stress increase is not used.

3 Limiting heights are based on continuous lateral support of each flange over the full height of the stud.

4 Listed limiting heights are based on steel properties only.

5 End reactions must be checked for web crippling separately.

6 Web crippling check based on 1-inch end bearing. Where limiting heights are followed by "e", web stiffeners are required.

7 Allowable moment is the lesser of local and distortional buckling. Stud distortional buckling based on an assumed $K\phi = 0$.

8 Members marked with an [1] have h/t > 200, and thus require end stiffeners.

9 Capacities are calculated according to the AISI S100-16 (2020) w/S2-20. A 1-1/2" by 4" knockout spaced no closer than 24" o.c. is assumed. (3/4" for 2-1/2" studs).

10 All values are based on Fy=33ksi for 33mil and 43mil Studs, and Fy=50ksi for 54mil, 68mil and 97mil Studs.

11 For deflection calculations, interior wall loads have been multiplied by 1.0 per AISI S240.

INTERIOR WALL HEIGHTS

With structural framing

3-5/8" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
362S137-33	12	23' 3"	18' 5"	16' 1"
	16	21' 1"	16' 9"	14' 8"
	24	17' 6"	14' 8"	12' 10"
362S137-43	12	25' 3"	20' 1"	17' 6"
	16	23' 0"	18' 3"	15' 11"
	24	20' 1"	15' 11"	13' 11"
362S137-54	12	27' 1"	21' 6"	18' 9"
	16	24' 7"	19' 6"	17' 1"
	24	21' 6"	17' 1"	14' 11"
362S137-68	12	28' 11"	22' 11"	20' 1"
	16	26' 3"	20' 10"	18' 3"
	24	22' 11"	18' 3"	15' 11"
362S137-97	12	31' 10"	25' 3"	22' 1"
	16	28' 11"	22' 11"	20' 1"
	24	25' 3"	20' 1"	17' 6"
362S162-33	12	24' 4"	19' 4"	16' 11"
	16	22' 2"	17' 7"	15' 4"
	24	18' 9"	15' 4"	13' 5"
362S162-43	12	26' 6"	21' 0"	18' 5"
	16	24' 1"	19' 1"	16' 8"
	24	21' 0"	16' 8"	14' 7"
362S162-54	12	28' 5"	22' 6"	19' 8"
	16	25' 10"	20' 6"	17' 11"
	24	22' 6"	17' 11"	15' 7"
362S162-68	12	30' 5"	24' 1"	21' 1"
	16	27' 7"	21' 11"	19' 2"
	24	24' 1"	19' 2"	16' 9"
362S162-97	12	33' 6"	26' 7"	23' 3"
	16	30' 5"	24' 2"	21' 1"
	24	26' 7"	21' 1"	18' 5"

3-5/8" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
362S200-33	12	25' 8"	20' 4"	17' 9"
	16	23' 3"	18' 6"	16' 2"
	24	19' 8"	16' 2"	14' 1"
362S200-43	12	28' 0"	22' 3"	19' 5"
	16	25' 5"	20' 2"	17' 8"
	24	22' 3"	17' 8"	15' 5"
362S200-54	12	30' 0"	23' 10"	20' 10"
	16	27' 3"	21' 8"	18' 11"
	24	23' 10"	18' 11"	16' 6"
362S200-68	12	32' 2"	25' 6"	22' 3"
	16	29' 2"	23' 2"	20' 3"
	24	25' 6"	20' 3"	17' 8"
362S200-97	12	35' 6"	28' 3"	24' 8"
	16	32' 3"	25' 8"	22' 5"
	24	28' 3"	22' 5"	19' 7"
362S250-43	12	29' 6"	23' 5"	20' 6"
	16	26' 10"	21' 3"	18' 7"
	24	23' 5"	18' 7"	16' 3"
362S250-54	12	31' 7"	25' 1"	21' 11"
	16	28' 8"	22' 9"	19' 11"
	24	25' 1"	19' 11"	17' 4"
362S250-68	12	33' 11"	26' 11"	23' 6"
	16	30' 10"	24' 6"	21' 5"
	24	26' 11"	21' 5"	18' 8"
362S250-97	12	37' 7"	29' 10"	26' 1"
	16	34' 2"	27' 1"	23' 8"
	24	29' 10"	23' 8"	20' 8"

Notes:

1 Studs are checked for simple-span deflection and stress. Stress calculations are made for mid-span fully braced moment, end shear through the unperforated section and shear moment interaction through the perforated section 10" away from the end bearing.

2 A 1/3 stress increase is not used.

3 Limiting heights are based on continuous lateral support of each flange over the full height of the stud.

4 Listed limiting heights are based on steel properties only.

5 End reactions must be checked for web crippling separately.

6 Web crippling check based on 1-inch end bearing. Where limiting heights are followed by "e", web stiffeners are required.

7 Allowable moment is the lesser of local and distortional buckling. Stud distortional buckling based on an assumed $K\phi = 0$.

8 Members marked with an [1] have h/t > 200, and thus require end stiffeners.

9 Capacities are calculated according to the AISI S100-16 (2020) w/S2-20. A 1-1/2" by 4" knockout spaced no closer than 24" o.c. is assumed. (3/4" for 2-1/2" studs).

10 All values are based on Fy=33ksi for 33mil and 43mil Studs, and Fy=50ksi for 54mil, 68mil and 97mil Studs.

11 For deflection calculations, interior wall loads have been multiplied by 1.0 per AISI S240.

Complies with AISI S100-16 (2020) w/S2-20 • IBC 2021

INTERIOR WALL HEIGHTS

With structural framing

4" Structural Framing

Member	Spacing (in) o.c.	5psf		
		L/120	L/240	L/360
400S137-33	12	25' 1"	19' 11"	17' 5"
	16	22' 7"	18' 1"	15' 10"
	24	18' 6"	15' 10"	13' 10"
400S137-43	12	27' 4"	21' 8"	18' 11"
	16	24' 10"	19' 8"	17' 2"
	24	21' 8"	17' 2"	15' 0"
400S137-54	12	29' 3"	23' 2"	20' 3"
	16	26' 7"	21' 1"	18' 5"
	24	23' 2"	18' 5"	16' 1"
400S137-68	12	31' 3"	24' 10"	21' 8"
	16	28' 5"	22' 7"	19' 8"
	24	24' 10"	19' 8"	17' 2"
400S137-97	12	34' 5"	27' 4"	23' 11"
	16	31' 3"	24' 10"	21' 8"
	24	27' 4"	21' 8"	18' 11"
400S162-33	12	26' 3"	20' 10"	18' 3"
	16	23' 11"	18' 11"	16' 7"
	24	19' 10"	16' 7"	14' 6"
400S162-43	12	28' 7"	22' 8"	19' 10"
	16	26' 0"	20' 7"	18' 0"
	24	22' 8"	18' 0"	15' 9"
400S162-54	12	30' 8"	24' 4"	21' 3"
	16	27' 10"	22' 1"	19' 4"
	24	24' 4"	19' 4"	16' 10"
400S162-68	12	32' 10"	26' 0"	22' 9"
	16	29' 10"	23' 8"	20' 8"
	24	26' 0"	20' 8"	18' 1"
400S162-97	12	36' 3"	28' 9"	25' 1"
	16	32' 11"	26' 1"	22' 10"
	24	28' 9"	22' 10"	19' 11"

4" Structural Framing

Member	Spacing (in) o.c.	5psf		
		L/120	L/240	L/360
400S200-33	12	27' 8"	21' 11"	19' 2"
	16	25' 1"	19' 11"	17' 5"
	24	20' 10"	17' 5"	15' 2"
400S200-43	12	30' 2"	23' 11"	20' 11"
	16	27' 5"	21' 9"	19' 0"
	24	23' 11"	19' 0"	16' 7"
400S200-54	12	32' 4"	25' 8"	22' 5"
	16	29' 5"	23' 4"	20' 5"
	24	25' 8"	20' 5"	17' 10"
400S200-68	12	34' 8"	27' 6"	24' 0"
	16	31' 6"	25' 0"	21' 10"
	24	27' 6"	21' 10"	19' 1"
400S200-97	12	38' 5"	30' 6"	26' 7"
	16	34' 10"	27' 8"	24' 2"
	24	30' 6"	24' 2"	21' 1"
400S250-43	12	31' 9"	25' 3"	22' 0"
	16	28' 10"	22' 11"	20' 0"
	24	25' 3"	20' 0"	17' 6"
400S250-54	12	34' 0"	27' 0"	23' 7"
	16	30' 10"	24' 6"	21' 5"
	24	27' 0"	21' 5"	18' 8"
400S250-68	12	36' 7"	29' 0"	25' 4"
	16	33' 3"	26' 4"	23' 0"
	24	29' 0"	23' 0"	20' 1"
400S250-97	12	40' 7"	32' 2"	28' 1"
	16	36' 10"	29' 3"	25' 7"
	24	32' 2"	25' 7"	22' 4"

Notes:

1 Studs are checked for simple-span deflection and stress. Stress calculations are made for mid-span fully braced moment, end shear through the unperforated section and shear moment interaction through the perforated section 10" away from the end bearing.

2 A 1/3 stress increase is not used.

3 Limiting heights are based on continuous lateral support of each flange over the full height of the stud.

4 Listed limiting heights are based on steel properties only.

5 End reactions must be checked for web crippling separately.

6 Web crippling check based on 1-inch end bearing. Where limiting heights are followed by "e", web stiffeners are required.

7 Allowable moment is the lesser of local and distortional buckling. Stud distortional buckling based on an assumed $K\phi = 0$.

8 Members marked with an [1] have h/t > 200, and thus require end stiffeners.

9 Capacities are calculated according to the AISI S100-16 (2020) w/S2-20. A 1-1/2" by 4" knockout spaced no closer than 24" o.c. is assumed. (3/4" for 2-1/2" studs).

10 All values are based on Fy=33ksi for 33mil and 43mil Studs, and Fy=50ksi for 54mil, 68mil and 97mil Studs.

11 For deflection calculations, interior wall loads have been multiplied by 1.0 per AISI S240.

INTERIOR WALL HEIGHTS

With structural framing

5-1/2" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
550S137-33	12	31' 7"	25' 7"	22' 5"
	16	27' 4"	23' 3"	20' 4"
	24	22' 4"	20' 4"	17' 9"
550S137-43	12	35' 2"	27' 11"	24' 4"
	16	31' 11"	25' 4"	22' 2"
	24	26' 9"	22' 2"	19' 4"
550S137-54	12	37' 8"	29' 11"	26' 1"
	16	34' 3"	27' 2"	23' 9"
	24	29' 11"	23' 9"	20' 9"
550S137-68	12	40' 4"	32' 0"	28' 0"
	16	36' 8"	29' 1"	25' 5"
	24	32' 0"	25' 5"	22' 2"
550S137-97	12	44' 7"	35' 5"	30' 11"
	16	40' 6"	32' 2"	28' 1"
	24	35' 5"	28' 1"	24' 6"
550S162-33	12	33' 8"	26' 9"	23' 4"
	16	29' 5"	24' 4"	21' 3"
	24	24' 0"	21' 3"	18' 6"
550S162-43	12	36' 8"	29' 1"	25' 5"
	16	33' 4"	26' 5"	23' 1"
	24	29' 1"	23' 1"	20' 2"
550S162-54	12	39' 4"	31' 3"	27' 3"
	16	35' 9"	28' 5"	24' 9"
	24	31' 3"	24' 9"	21' 8"
550S162-68	12	42' 2"	33' 6"	29' 3"
	16	38' 4"	30' 5"	26' 7"
	24	33' 6"	26' 7"	23' 3"
550S162-97	12	46' 9"	37' 1"	32' 5"
	16	42' 5"	33' 8"	29' 5"
	24	37' 1"	29' 5"	25' 8"

5-1/2" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
550S200-33	12	35' 4"	28' 0"	24' 6"
	16	31' 4"	25' 5"	22' 3"
	24	25' 7"	22' 3"	19' 5"
550S200-43	12	38' 7"	30' 7"	26' 9"
	16	35' 1"	27' 10"	24' 4"
	24	30' 6"	24' 4"	21' 3"
550S200-54	12	41' 5"	32' 10"	28' 8"
	16	37' 7"	29' 10"	26' 1"
	24	32' 10"	26' 1"	22' 9"
550S200-68	12	44' 5"	35' 3"	30' 10"
	16	40' 4"	32' 0"	28' 0"
	24	35' 3"	28' 0"	24' 5"
550S200-97	12	49' 3"	39' 1"	34' 2"
	16	44' 9"	35' 6"	31' 1"
	24	39' 1"	31' 1"	27' 1"
550S250-43	12	40' 5"	32' 1"	28' 1"
	16	36' 9"	29' 2"	25' 6"
	24	31' 4"	25' 6"	22' 3"
550S250-54	12	43' 3"	34' 4"	30' 0"
	16	39' 3"	31' 2"	27' 3"
	24	34' 4"	27' 3"	23' 10"
550S250-68	12	46' 7"	37' 0"	32' 4"
	16	42' 4"	33' 7"	29' 4"
	24	37' 0"	29' 4"	25' 8"
550S250-97	12	51' 10"	41' 2"	35' 11"
	16	47' 1"	37' 4"	32' 8"
	24	41' 2"	32' 8"	28' 6"

Notes:

1 Studs are checked for simple-span deflection and stress. Stress calculations are made for mid-span fully braced moment, end shear through the unperforated section and shear moment interaction through the perforated section 10" away from the end bearing.

2 A 1/3 stress increase is not used.

3 Limiting heights are based on continuous lateral support of each flange over the full height of the stud.

4 Listed limiting heights are based on steel properties only.

5 End reactions must be checked for web crippling separately.

6 Web crippling check based on 1-inch end bearing. Where limiting heights are followed by "e", web stiffeners are required.

7 Allowable moment is the lesser of local and distortional buckling. Stud distortional buckling based on an assumed $K\phi = 0$.

8 Members marked with an [1] have h/t > 200, and thus require end stiffeners.

9 Capacities are calculated according to the AISI S100-16 (2020) w/S2-20. A 1-1/2" by 4" knockout spaced no closer than 24" o.c. is assumed. (3/4" for 2-1/2" studs).

10 All values are based on Fy=33ksi for 33mil and 43mil Studs, and Fy=50ksi for 54mil, 68mil and 97mil Studs.

11 For deflection calculations, interior wall loads have been multiplied by 1.0 per AISI S240.

Complies with AISI S100-16 (2020) w/S2-20 • IBC 2021

INTERIOR WALL HEIGHTS

With structural framing

6" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
600S137-33	12	33' 1"	27' 3"	23' 10"
	16	28' 7"	24' 9"	21' 8"
	24	23' 4"	21' 8"	18' 11"
600S137-43	12	37' 8"	29' 11"	26' 2"
	16	34' 3"	27' 2"	23' 9"
	24	28' 1"	23' 9"	20' 9"
600S137-54	12	40' 5"	32' 1"	28' 0"
	16	36' 9"	29' 2"	25' 6"
	24	32' 1"	25' 6"	22' 3"
600S137-68	12	43' 4"	34' 4"	30' 0"
	16	39' 4"	31' 3"	27' 3"
	24	34' 4"	27' 3"	23' 10"
600S137-97	12	47' 11"	38' 0"	33' 2"
	16	43' 6"	34' 6"	30' 2"
	24	38' 0"	30' 2"	26' 4"
600S162-33	12	35' 6"	28' 8"	25' 0"
	16	30' 9"	26' 0"	22' 9"
	24	25' 2"	22' 9"	19' 10"
600S162-43	12	39' 4"	31' 2"	27' 3"
	16	35' 9"	28' 4"	24' 9"
	24	31' 1"	24' 9"	21' 8"
600S162-54	12	42' 2"	33' 6"	29' 3"
	16	38' 4"	30' 5"	26' 7"
	24	33' 6"	26' 7"	23' 3"
600S162-68	12	45' 3"	35' 11"	31' 4"
	16	41' 1"	32' 7"	28' 6"
	24	35' 11"	28' 6"	24' 11"
600S162-97	12	50' 1"	39' 9"	34' 9"
	16	45' 6"	36' 2"	31' 7"
	24	39' 9"	31' 7"	27' 7"

6" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
600S200-33	12	37' 9"	30' 0"	26' 2"
	16	32' 10"	27' 3"	23' 10"
	24	26' 10"	23' 10"	20' 10"
600S200-43	12	41' 3"	32' 9"	28' 7"
	16	37' 6"	29' 9"	26' 0"
	24	32' 0"	26' 0"	22' 9"
600S200-54	12	44' 4"	35' 2"	30' 9"
	16	40' 3"	32' 0"	27' 11"
	24	35' 2"	27' 11"	24' 5"
600S200-68	12	47' 7"	37' 9"	33' 0"
	16	43' 2"	34' 4"	29' 11"
	24	37' 9"	29' 11"	26' 2"
600S200-97	12	52' 10"	41' 11"	36' 7"
	16	48' 0"	38' 1"	33' 3"
	24	41' 11"	33' 3"	29' 1"
600S250-43	12	43' 3"	34' 4"	30' 0"
	16	39' 3"	31' 2"	27' 3"
	24	32' 11"	27' 3"	23' 10"
600S250-54	12	46' 3"	36' 8"	32' 1"
	16	42' 0"	33' 4"	29' 1"
	24	36' 8"	29' 1"	25' 5"
600S250-68	12	49' 10"	39' 7"	34' 7"
	16	45' 3"	35' 11"	31' 5"
	24	39' 7"	31' 5"	27' 5"
600S250-97	12	55' 5"	44' 0"	38' 5"
	16	50' 4"	40' 0"	34' 11"
	24	44' 0"	34' 11"	30' 6"

Notes:

1 Studs are checked for simple-span deflection and stress. Stress calculations are made for mid-span fully braced moment, end shear through the unperforated section and shear moment interaction through the perforated section 10" away from the end bearing.

2 A 1/3 stress increase is not used.

3 Limiting heights are based on continuous lateral support of each flange over the full height of the stud.

4 Listed limiting heights are based on steel properties only.

5 End reactions must be checked for web crippling separately.

6 Web crippling check based on 1-inch end bearing. Where limiting heights are followed by "e", web stiffeners are required.

7 Allowable moment is the lesser of local and distortional buckling. Stud distortional buckling based on an assumed $K\phi = 0$.

8 Members marked with an [1] have h/t > 200, and thus require end stiffeners.

9 Capacities are calculated according to the AISI S100-16 (2020) w/S2-20. A 1-1/2" by 4" knockout spaced no closer than 24" o.c. is assumed. (3/4" for 2-1/2" studs).

10 All values are based on Fy=33ksi for 33mil and 43mil Studs, and Fy=50ksi for 54mil, 68mil and 97mil Studs.

11 For deflection calculations, interior wall loads have been multiplied by 1.0 per AISI S240.

INTERIOR WALL HEIGHTS

With structural framing

8" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
800S137-33[1]	12	37' 10"	34' 0"	29' 8"
	16	32' 9"	30' 11"	27' 0"
	24	26' 9"	26' 9"	23' 7"
800S137-43	12	45' 11"	37' 5"	32' 8"
	16	39' 9"	34' 0"	29' 9"
	24	32' 5"	29' 9"	25' 11"
800S137-54	12	50' 9"	40' 3"	35' 2"
	16	46' 1"	36' 7"	31' 11"
	24	40' 3"	31' 11"	27' 11"
800S137-68	12	54' 10"	43' 6"	38' 0"
	16	49' 10"	39' 6"	34' 6"
	24	43' 6"	34' 6"	30' 2"
800S137-97	12	60' 10"	48' 4"	42' 2"
	16	55' 4"	43' 11"	38' 4"
	24	48' 4"	38' 4"	33' 6"
800S162-33[1]	12	41' 0"	35' 5"	30' 11"
	16	35' 6"	32' 2"	28' 1"
	24	29' 0" e	28' 1"	24' 7"
800S162-43	12	49' 1"	38' 11"	34' 0"
	16	42' 10"	35' 4"	30' 11"
	24	35' 0"	30' 11"	27' 0"
800S162-54	12	52' 9"	41' 10"	36' 7"
	16	47' 11"	38' 1"	33' 3"
	24	41' 10"	33' 3"	29' 0"
800S162-68	12	57' 0"	45' 3"	39' 6"
	16	51' 10"	41' 1"	35' 11"
	24	45' 3"	35' 11"	31' 5"
800S162-97	12	63' 5"	50' 4"	43' 11"
	16	57' 7"	45' 9"	39' 11"
	24	50' 4"	39' 11"	34' 11"

8" Structural Framing

Member	Spacing (in) o.c.	5psf L/120	5psf L/240	5psf L/360
800S200-33[1]	12	44' 0"	37' 9"	33' 0"
	16	38' 1"	34' 3"	29' 11"
	24	31' 1" e	29' 11" e	26' 2"
800S200-43	12	51' 10"	41' 1"	35' 11"
	16	45' 10"	37' 4"	32' 8"
	24	37' 5"	32' 8"	28' 6"
800S200-54	12	55' 8"	44' 2"	38' 7"
	16	50' 7"	40' 2"	35' 1"
	24	44' 2"	35' 1"	30' 8"
800S200-68	12	59' 9"	47' 5"	41' 5"
	16	54' 4"	43' 1"	37' 8"
	24	47' 5"	37' 8"	32' 11"
800S200-97	12	66' 6"	52' 9"	46' 1"
	16	60' 5"	47' 11"	41' 11"
	24	52' 9"	41' 11"	36' 7"
800S250-43	12	54' 0"	42' 11"	37' 6"
	16	47' 0"	39' 0"	34' 0"
	24	38' 4"	34' 0"	29' 9"
800S250-54	12	57' 10"	45' 11"	40' 1"
	16	52' 7"	41' 8"	36' 5"
	24	45' 11"	36' 5"	31' 10"
800S250-68	12	62' 4"	49' 6"	43' 3"
	16	56' 8"	44' 11"	39' 3"
	24	49' 6"	39' 3"	34' 4"
800S250-97	12	69' 6"	55' 2"	48' 2"
	16	63' 2"	50' 1"	43' 9"
	24	55' 2"	43' 9"	38' 3"

Notes:

1 Studs are checked for simple-span deflection and stress. Stress calculations are made for mid-span fully braced moment, end shear through the unperforated section and shear moment interaction through the perforated section 10" away from the end bearing.

2 A 1/3 stress increase is not used.

3 Limiting heights are based on continuous lateral support of each flange over the full height of the stud.

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10 All values are based on Fy=33ksi for 33mil and 43mil Studs, and Fy=50ksi for 54mil, 68mil and 97mil Studs.

11 For deflection calculations, interior wall loads have been multiplied by 1.0 per AISI S240.

Complies with AISI S100-16 (2020) w/S2-20 • IBC 2021